

Structural Calculation & Analysis For Sunforson system



Wind load calculation and weight ballasted

Wind load	Tilt angle	Panel size
35m/s	25 °	2279*1134*35

Wind load

The design wind speed is 35m/s, according to BS 6399-2:1997 the value of the dynamic pressure q_s

Wind load: $q_s = 35 \times 35 \times 0.613 \text{ Pa} = 751 \text{ Pa}$

The size effect factor for external pressures: $C_a = 0.952$ Refer to figure 4 in BS 6399-2

Refer to Table 13 in BS 6399-2, Take 25 ° as design degree

The net pressure overall coefficients: $C_{pe+} = 1, C_{pe-} = 0.8$

The pressure: $p_e (+) = C_a \times C_{pe} \times q_s = 715 \text{ Pa},$

$p_e (-) = C_a \times C_{pe} \times q_s = 572 \text{ Pa}$

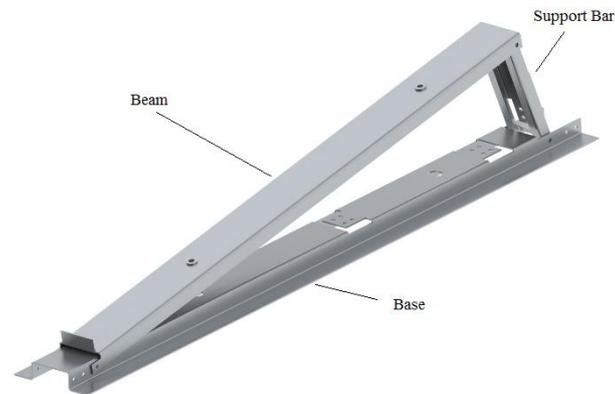
Modules: $W_g = 280 \text{ N}$

Length of Modules: $L_a = 2279 \text{ mm}$

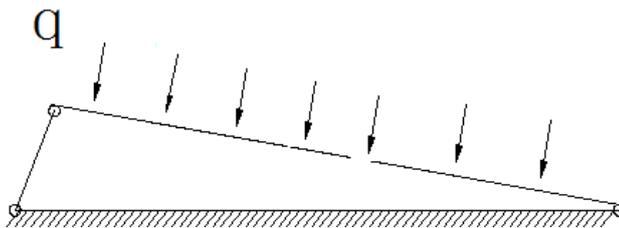
Width of Modules: $L_b = 1134 \text{ mm}$

Area of Modules: $A_o = L_a \times L_b = 2.59 \text{ m}^2, P_{wg} = W_g / A_o = 108 \text{ N/m}^2$

The SFS-FR-02 SunFix Components:



1. Check the Triangular Mounts Group



1) Beam

Material of clamp: 6005- T5

Design strength: $\sigma_s=240\text{MPa}$

Design shear strength: $\tau_{av}=205\text{MPa}$

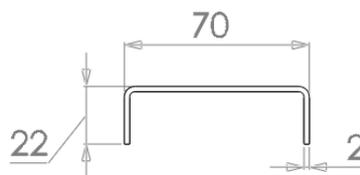
Area of section: $A_2= 214.85\text{ m m}^2$

Second Moment Of Area:

$I_x = 8762.46\text{ mm}^4$ $I_y = 143500.67\text{ mm}^4$

Elastic Modulus:

$W_x = 796.58\text{mm}^3$ $W_y=4100.02\text{mm}^3$

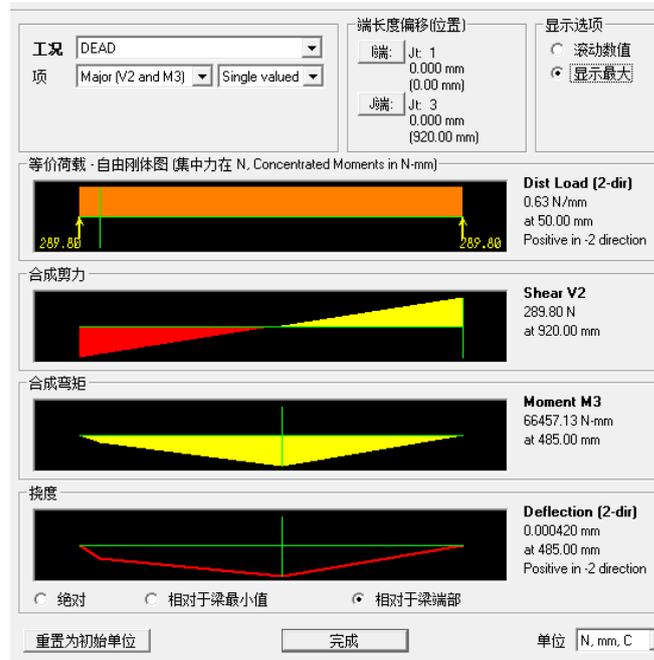


$$q=Pe (+) /Lb=0.63\text{N}/\text{mm}^2$$

used SAP2000 to analysis

The results are as follows

Main beam



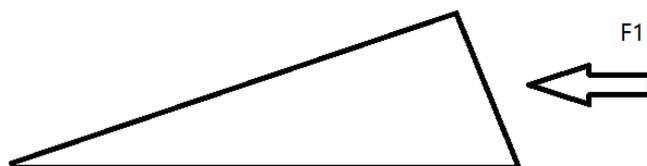
The Max moment is $M_{max}=66457N.m$

The $\sigma_{max} = M_{max} / W_x = 60Mpa < pot5=240MPa$

So, load capacity is satisfied

2.Check the load weight

Area of Modules: $A_0 = L_b * L_a = 2.59m^2$



Total Wind load: $F_1 = P_e(-) * A_0 * \sin 25 = 626N$

PV weigh $W_g = 280N$

The weight of the concrete pier $T = 2250N$

The coeficeon of friction between rooftop surface and aluminum alloy is: $N = 0.25$

Capricorn of ballasted blocks (m) and system self weight: $f = (T + Z) * N \geq F_1$